

14 HYDRODYNAMIC FORCES ON VERTICAL CYLINDERS  
15 AND THE LIGHTHILL CORRECTION

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18 **Abstract**—In the keynote address to the 1979 Behaviour of Offshore Structures (BOSS)  
19 Conference, Sir James Lighthill pointed out the absence of a second-order term of potential  
20 origin from the Morison description of the hydrodynamic force on a vertical cylinder. This  
21 term, referred to as the Lighthill correction, is due to the nonlinear interaction between the  
22 flow velocity and its horizontal gradient. As noted by Lighthill, if this term is omitted, the  
23 estimated drag force in the Morison equation is equal in effect to the actual drag force *plus*  
24 the Lighthill correction.

25 Thus, it would appear that in cases where hydrodynamic damping plays an important role  
26 and should therefore be estimated as accurately as possible, corrections of the Lighthill type  
27 might have to be added to the Morison expression for the hydrodynamic force. (One such case  
28 is the dynamic amplification of wind-induced fluctuating motions of tension leg platforms.) In  
29 particular, it might be expected that the estimation of the damping force would be more strongly  
30 affected in situations involving low Keulegan-Carpenter numbers, and therefore relatively low  
31 damping forces.

32 It is thus of interest to examine the effect of the Lighthill correction quantitatively. In this  
33 work, the expression for the Lighthill correction was derived for finite water depths.  
34 Measurements obtained in periodic wave flow at the Naval Civil Engineering Laboratory and  
35 in random wave flow at the Delft Hydraulics Laboratory were subjected to an extensive analysis.  
36 The results of the analysis showed that for both the periodic and random wave conditions and  
37 addition of the Lighthill correction (1) did not improve the Morison equation significantly, and  
38 (2) had no significant effect on the estimation of the drag force, including the drag force  
39 corresponding to very low Keulegan-Carpenter numbers.  
40

41 INTRODUCTION

42 WAVE FORCES on cylindrical elements are of considerable interest in the design of  
43 offshore facilities. Morison *et al.* (1950) proposed a simple equation expressing the total  
44 wave force as the sum of two components: an inertia force, due to the effects of  
45 irrotational (potential) flow, and a drag force, due to viscosity (skin friction and flow  
46 separation) effects. The equation is calibrated with two empirical coefficients which are  
47 referred to as the inertia and drag coefficient and are functions of the flow conditions.

48 The Morison equation has been criticized as oversimplifying the fluid mechanics of  
49 the loading but an alternative rigorous approach has not been developed to date. There  
50 appears to be a consensus that, to represent the fluid mechanics more closely, it is  
51 better to add correction terms to the Morison equation rather than devise a completely  
52 new relationship (Keulegan and Carpenter, 1958; Lighthill, 1979; Sarpkaya, 1981; Cook,  
53 1987). The corrections of Keulegan, Carpenter and Sarpkaya are aimed essentially at  
54 accounting for vorticity effects. The topic of this paper, the Lighthill correction, is a  
55 correction associated with irrotational (potential) flow effects.

56 In his keynote address to the 1979 Conference on the Behaviour of Offshore  
57 Structures (BOSS) Sir James Lighthill showed that the force associated with the  
58 irrotational flow includes, in addition to the linear inertia term of the Morison equation,

a nonlinear effect of potential origin due to the extensional motion (that is, the horizontal gradient of the in-line component of the flow velocity). Lighthill also noted that if the total force on a cylinder is expressed as the sum of the two Morison equation terms only, then the Lighthill force, which is due to potential flow effects, is automatically incorporated into the nonlinear drag term. This drag term is purportedly due solely to viscosity effects. Therefore, the Morison equation leads to an erroneous estimation of the force due to viscosity. The degree to which the error is significant depends upon the ratio between the Lighthill force and the actual Morison component associated with viscosity effects. This latter component is responsible for the bulk of damping that controls the dynamic response of compliant offshore structures to fluctuating wind (Simiu and Leigh, 1983; Cook *et al.*, 1986). The question of the extent to which corrections of the Lighthill type might affect the estimation of this component is therefore of significant practical interest in this context.

The primary objective of this paper is to investigate the significance of the Lighthill correction in quantitative terms. Two sets of data were used for the purpose of investigating the quantitative significance of the Lighthill correction. The first set was provided by the Naval Civil Engineering Laboratory (NCEL), and consisted of periodic flow force and flow measurements obtained in a wave tank (see Hudspeth and Nath (1985) for details). The second set was provided by the Delft Hydraulics Laboratory (DHL) and consisted of force and flow measurements obtained in a wave tank under random wave flow conditions (see Bearman *et al.* (1985a) for details).

#### THE MORISON EQUATION AND THE LIGHTHILL CORRECTION

The Morison equation (Morison *et al.*, 1950) is widely used in ocean engineering as an expression for wave-induced forces on structural members. For the case of a circular cylinder of diameter  $D$ , the Morison equation is usually expressed as

$$F = \frac{1}{4} \rho \pi D^2 C_m \frac{du}{dt} + \frac{1}{2} \rho D C_d u |u| \quad (1)$$

where  $F$  is the force per unit length,  $\rho$  is the fluid density,  $u$  is the undisturbed fluid velocity and  $C_d$  and  $C_m$  are the drag and inertia coefficients, respectively. With the widespread use of the Morison equation a great deal of work has been done on evaluating the appropriate values of the force coefficients. A review of this work is presented in Sarpkaya and Isaacson (1981).

As noted by Lighthill (1979) the fluid motion around a structure can be viewed as being due to (1) an irrotational flow that satisfies the boundary conditions, and (2) a vortex motion associated with any vorticity that has been shed (and satisfies zero boundary conditions, that is, zero fluid motion far from the body). It is the component due to the irrotational flow that Lighthill considers.

Lighthill derived two main second-order correction terms to the Morison equation. The corrections are due to the nonlinear interaction between a surface piercing cylinder and the irrotational flow field. The flow was assumed to consist of sinusoidal waves propagating in the positive  $x$  direction. The first correction term is a waterline force due to integration of the pressure between the still water level and the instantaneous free surface. If a body is totally submerged, as in the case of a horizontal cylinder or of nonsurface-piercing elements then this waterline force is not present. The second of

the correction terms is due to the horizontal gradient of the velocity (the extensional motion) and is given by the resultant of the dynamic pressure acting over the body's surface. Owing to the nature of the data being analysed in this paper we will consider the second correction only.

At any point in a fluid, if the velocity potential  $\phi$  is known the fluid pressure at any point is determined by Bernoulli's equation

$$p = -p_s - \rho g z - \rho \frac{\partial \phi}{\partial t} - \frac{1}{2} \rho (\nabla \phi)^2 + C(t) \quad (2)$$

where  $\rho$  is the fluid density,  $z$  is the distance from the still water level to the point being considered,  $g$  is the acceleration due to gravity,  $C(t)$  is a function independent of the coordinates and  $p_s$  is the atmospheric pressure. Both  $p_s$  and  $C(t)$  may be taken equal to zero without loss of generality, see Stoker (1977).

Expanding the pressure  $p$  and the right-hand side of Equation (2) with respect to a perturbation parameter  $\epsilon$  ( $\epsilon$  is the wave steepness and equals  $ak$ , where  $a$  is the wave amplitude and  $k$  is the wavenumber) and equating powers of  $\epsilon^2$  we obtain the second order pressure as

$$p_2 = -\rho \frac{\partial \phi_2}{\partial t} - \frac{1}{2} \rho (\nabla \phi_1)^2. \quad (3)$$

We seek the expression for  $\phi_1$  and  $\phi_2$  for the wave flow as modified by the presence of the cylinder. We first consider a potential flow with velocity  $u$  (in potential theory  $u = \partial \phi / \partial x$ ) in the far field. The presence of a circular cylinder results in a flow field whose potential  $\phi_d$  corresponds to a dipole (Milne-Thompson, 1960; p. 154), that is

$$\phi_d = u \left( r + \frac{b^2}{r} \right) \cos \theta \quad (4)$$

where  $r$  is the radius to the point being considered,  $b$  is the cylinder radius and  $\theta$  is the angle between the axis and the point being considered. If  $\phi = \phi_d$  integration around the cylinder of the first term in Equation (3) yields the second order inertia force.

Setting  $\phi$  equal to  $\phi_d$  would be sufficient if the cylinder response was due to a fluctuating velocity only. However, in the case of a wave flow the in-line velocity has a nonzero horizontal gradient (extension) denoted by  $E = \partial u / \partial x$ . The extension can be expressed as a sum of a pure dilatation and a dilatationless strain (Lighthill, 1979). The cylinder responds to the variable extension because the cylinder itself impedes the local extensional motion of the fluid. This leads to a local compensating addition to the irrotational flow field, whose potential may be expressed as the sum of two terms:

(a) a monopole field associated with the pure dilatation to which there corresponds the potential,  $\phi_m$ , equal to (Lighthill, 1979; p. 19).

$$\phi_m = \frac{E}{4} \left( r^2 - 2b^2 \ln \frac{r}{b} \right) \quad (5)$$

where  $E$  is the extension and the other notations are the same as in Equation (4).

(b) a quadrupole field associated with the dilatationless strain to which there corresponds the potential,  $\phi_q$ , equal to (Paterson, 1983; p. 217)

$$\phi_d = \frac{E}{4} \left( r^2 + \frac{b^4}{r^2} \right) \cos 2\theta \quad (6)$$

where the notation is the same as in Equations (4) and (5). Therefore, the total potential for the fluctuating extension is given as the sum of Equations (5) and (6), that is,

$$\phi_e = \frac{E}{4} \left( r^2 - 2b^2 \ln \frac{r}{b} \right) + \frac{E}{4} \left( r^2 + \frac{b^4}{r^2} \right) \cos 2\theta. \quad (7)$$

To include the response to the fluctuating extension in the dynamic pressure, the extension needs to be expanded in a power series

$$E = \epsilon E_1 + \epsilon^2 E_2 + \dots \quad (8)$$

where  $E_1 = \frac{\partial^2 \phi_1}{\partial x^2}$  and  $E_2 = \frac{\partial^2 \phi_2}{\partial x^2}$ .

The total extension potential,  $\phi_e$ , expanded in the power series (8) gives

$$\begin{aligned} \phi_e = \epsilon \left\{ \frac{E_1}{4} \left( r^2 - 2b^2 \ln \frac{r}{b} \right) + \frac{E_1}{4} \left( r^2 + \frac{b^4}{r^2} \right) \cos 2\theta \right\} \\ + \epsilon^2 \left\{ \frac{E_2}{4} \left( r^2 - 2b^2 \ln \frac{r}{b} \right) + \frac{E_2}{4} \left( r^2 + \frac{b^4}{r^2} \right) \cos 2\theta \right\}. \end{aligned} \quad (9)$$

The basic fluctuating velocity potential,  $\phi_d$ , can be expanded as

$$\phi_d = \epsilon u_1 \left( r + \frac{b^2}{r} \right) \cos \theta + \epsilon^2 u_2 \left( r + \frac{b^2}{r} \right) \cos \theta. \quad (10)$$

The total potential is equal to the sum of the dipole and extension potentials, that is  $\phi = \phi_d + \phi_e$ . Using polar coordinates and noting that  $u_1 = \partial \phi_1 / \partial x$  and  $u_2 = \partial \phi_2 / \partial x$  we obtain the horizontal and vertical velocities on the cylinder surface ( $r = b$ ):

$$\begin{aligned} v_\theta = \frac{1}{r} \frac{\partial \phi}{\partial \theta} = \frac{1}{r} \frac{\partial}{\partial \theta} (\epsilon \phi_1 + \epsilon^2 \phi_2) \\ = \epsilon \left( -E_1 b \sin 2\theta - 2 \frac{\partial \phi_1}{\partial x} \sin \theta \right) + \\ \epsilon^2 \left( -E_2 b \sin 2\theta - 2 \frac{\partial \phi_2}{\partial x} \sin \theta \right) \end{aligned} \quad (11)$$

and

$$\begin{aligned} v_z = \frac{\partial \phi}{\partial z} = \frac{\partial}{\partial z} (\epsilon \phi_1 + \epsilon^2 \phi_2) \\ = \epsilon \left( \frac{\partial \phi_1}{\partial z} + 2 \frac{\partial^2 \phi_1}{\partial x \partial z} b \cos \theta + \frac{1}{4} \frac{\partial E_1}{\partial z} b^2 + \frac{1}{2} \frac{\partial E_1}{\partial z} b^2 \cos 2\theta \right) \\ + \epsilon^2 \left( \frac{\partial \phi_2}{\partial z} + 2 \frac{\partial^2 \phi_2}{\partial x \partial z} b \cos \theta + \frac{1}{4} \frac{\partial E_2}{\partial z} b^2 + \frac{1}{2} \frac{\partial E_2}{\partial z} b^2 \cos 2\theta \right). \end{aligned} \quad (12)$$

We can now calculate the total second order dynamic pressure,  $p_{2d} = (1/2\rho(\nabla\phi_1)^2)$ ,

$$\begin{aligned} p_{2d} = -\frac{\rho}{2} \left\{ E_1^2 b^2 \sin^2 2\theta + 4 \frac{\partial \phi_1}{\partial x} E_1 b \sin 2\theta \sin \theta \right. \\ + 4 \left( \frac{\partial \phi_1}{\partial x} \right)^2 \sin^2 \theta + \left( \frac{\partial \phi_1}{\partial z} \right)^2 + 4 \frac{\partial \phi_1}{\partial z} \frac{\partial^2 \phi_1}{\partial x \partial z} b \cos \theta \\ + \frac{1}{2} \frac{\partial \phi_1}{\partial z} \frac{\partial E_1}{\partial z} b^2 + \frac{\partial \phi_1}{\partial z} \frac{\partial E_1}{\partial z} b^2 \cos 2\theta + 4 \left( \frac{\partial^2 \phi_1}{\partial x \partial z} \right)^2 b^2 \cos^2 \theta \\ + \frac{\partial^2 \phi_1}{\partial x \partial z} \frac{\partial E_1}{\partial z} b^3 \cos \theta + 2 \frac{\partial E_1}{\partial z} \frac{\partial^2 \phi_1}{\partial x \partial z} b^3 \cos \theta \cos 2\theta \\ + \frac{1}{16} \left( \frac{\partial E_1}{\partial z} \right)^2 b^4 + \frac{1}{4} \left( \frac{\partial E_1}{\partial z} \right)^2 b^4 \cos 2\theta \\ \left. + \frac{1}{4} \left( \frac{\partial E_1}{\partial z} \right)^2 b^4 \cos^2 2\theta \right\}. \end{aligned} \quad (13)$$

The dynamic force around the cylinder is calculated as follows

$$\begin{aligned} F_{dy} = \int \frac{1}{2} \rho (\nabla \phi_1)^2 n_x ds \\ = \rho \pi b^2 u_{1x} E_1 + 2\rho \pi b^2 u_{1z} \frac{\partial u_{1z}}{\partial x} + 2\rho \pi b^4 \frac{\partial u_{1z}}{\partial x} \frac{\partial E_1}{\partial z} \end{aligned} \quad (14)$$

where the integral is taken over the wetted perimeter,  $n_x$  is the  $x$  component of the unit normal pointing toward the body,  $u_{1x} = \partial \phi_1 / \partial x$  is the first order velocity in the  $x$  direction,  $E_1 = \partial^2 \phi_1 / \partial x^2$  is the first order extension in the  $x$  direction,  $u_{1z} = \partial \phi_1 / \partial z$  is the first order velocity in the  $z$  direction and the other notations are as given previously. The total force on the section being considered is

$$F_{2d} = \int F_{dy} dz \quad (15)$$

where the integral is taken between the top and bottom of the submerged element being considered. For slender cylinders the last term in Equation (14) turns out to be insignificant compared to the first two. For  $kh > \pi$ , using Stokes first order theory we obtain Lighthill's results for deep water waves, that is

$$u_{1x} E_1 \approx -u_{1z} \frac{\partial u_{1z}}{\partial x} \quad (16)$$

and

$$F_{dy} = -\rho \pi b^2 u_{1x} E_1. \quad (17)$$

The total force acting on an elemental section of a circular cylinder is now equal to the two Morison equation terms plus the second order Lighthill correction (Equation (14)). According to Lighthill the analysis for his correction is equally applicable in random wave fields. If the analysis is to be performed in the time domain then the velocity is simply the local fluctuating velocity and the extension is the local fluctuating extension in the wave field.

The extension  $E$  and the spatial derivative of the vertical velocity  $u_z$  and of  $E$ , which are needed in Equation (14) were not measured, since such measurements cannot be carried out in practice. The quantities,  $E_1$ ,  $\partial u_z / \partial x$  and  $\partial E_1 / \partial z$  were estimated from measured flow properties as follows. For the periodic flow use was made of relations based on Stokes second order wave theory (Cook, 1987). For the random flow the measured records were decomposed into Fourier series and the requisite quantities were obtained by differentiation of the terms of the series so obtained.

#### ANALYSIS OF THE PERIODIC DATA

This section deals with the results of the analysis of the periodic data based on the Morison equation alone and then considers the effect of including the Lighthill correction. The data obtained from NCEL included the wave profile, in-line force and velocity measurements for a reasonable range of wave heights and wave periods.

Least squares analyses were performed to obtain the time invariant coefficients  $C_d$  and  $C_m$  based on the Morison equation. Figure 1 plots the drag coefficients for each individual wave, showing their variation with Keulegan-Carpenter number ( $KC = uT/D$  and is the ratio of the measure of the path length of a fluid particle during a wave period  $T$ , to the body diameter  $D$ ). The drag coefficients show a large variability at low  $KC$  numbers. This may be expected because the drag term is small relative to the inertia term and instabilities occur in its calculation. Note that above  $KC \approx 4$  the drag term is more significant and shows little variation with increasing  $KC$  numbers. It is noted that the dependence of the drag coefficient on  $KC$  is similar to that reported by Sarpkaya (1976).

Figure 2 plots the inertia coefficient against the  $KC$  number. It can be seen that at low  $KC$  numbers ( $KC < 4$ ) the inertia coefficient is greater than the ideal potential

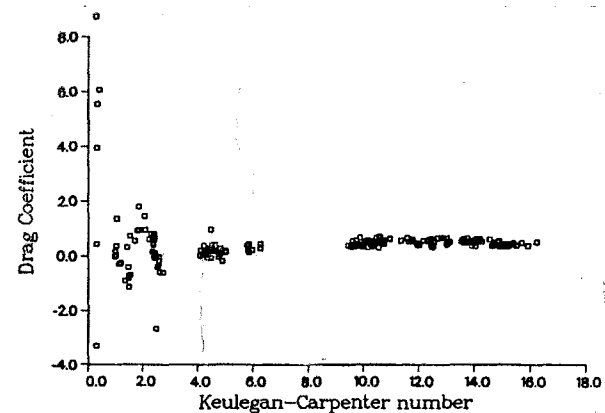


FIG. 1. Drag coefficient vs Keulegan-Carpenter number.

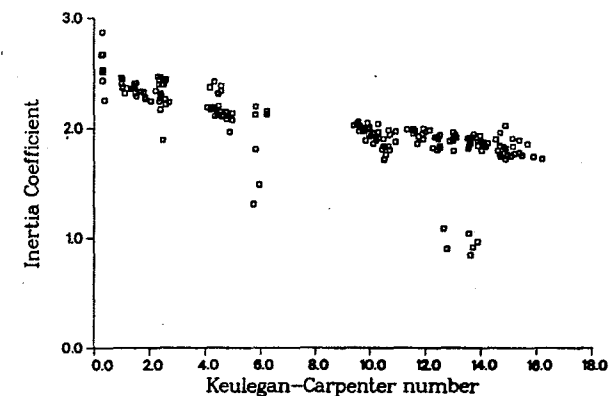


FIG. 2. Inertia coefficient vs Keulegan-Carpenter number.

flow value of 2.0. This is in agreement with results obtained by Chakrabarti *et al.* (1976) and Chakrabarti (1980) who also reported values of  $C_m$  significantly greater than 2. Note that  $C_m = 2$  if the effects of viscosity are neglected. Such effects are included in expressions for  $C_m$  given by Sarpkaya (1986) and Bearman *et al.* (1985b). However, these expressions are based on a linearization of the Navier-Stokes equations which is acceptable only if the frequencies of the wave motion are very high. This linearization is not applicable in the case of ocean or wave tank data.

Force time histories were calculated both for the full records and for each individual wave of the full record. The forces were calculated by assuming the validity of the Morison equation with time invariant coefficients. Figures 3-6, based on the analysis of the full record, show measured and calculated force time histories for the lowest  $KC$  ( $KC = 0.32$ ), as well as for  $KC = 4.41$ ,  $KC = 10.26$ , and the highest  $KC$  ( $KC = 15.31$ ), respectively. Also shown on these plots is the force residue, that is, the difference between the measured and the calculated force histories. In all figures pertaining to the periodic data, measured, and calculated forces are represented by solid and interrupted lines respectively, and force residues are represented by dash-dot lines.

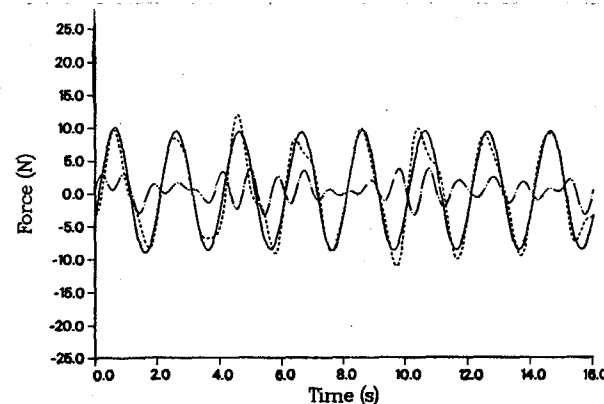


FIG. 3. Force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 0.32$ .

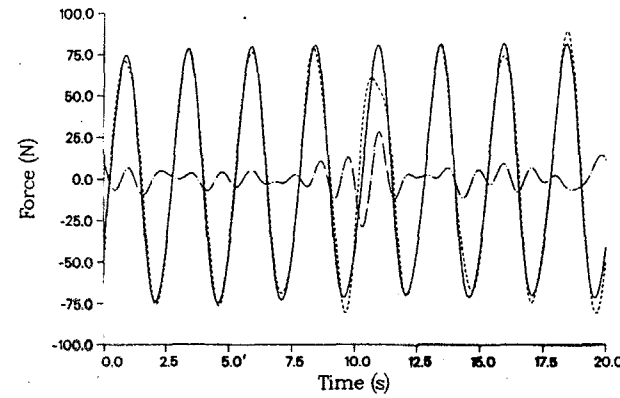


FIG. 4. Force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 4.41$ .

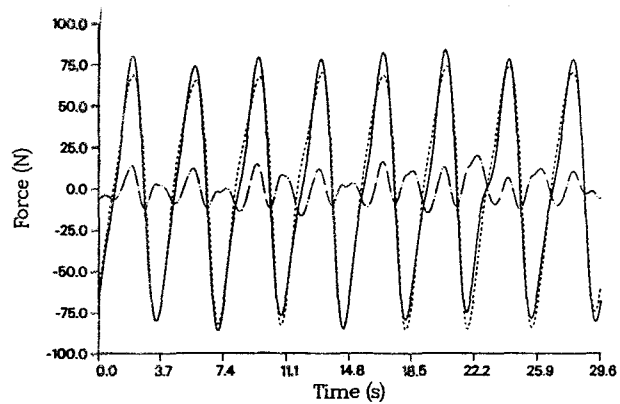


FIG. 5. Force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 10.26$ .

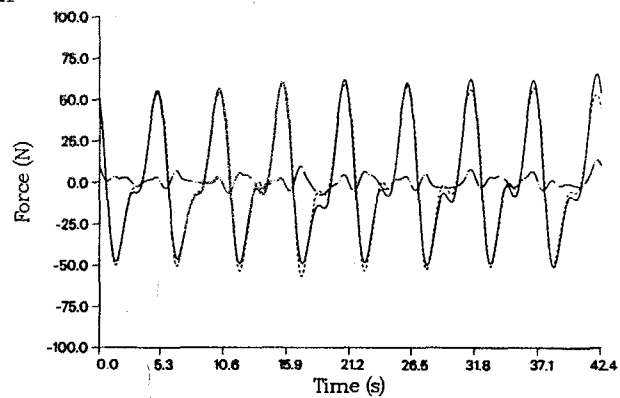


FIG. 6. Force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 15.31$ .

236 The dominant harmonic of the residue for all the force histories appears to be close to  
 237 the second harmonic of the force. It is noted that the Lighthill correction term also has  
 238 frequencies equal to twice the fundamental frequency.

239 Results corresponding to typical individual waves from each of the runs plotted in  
 240 Figs 3–6 are given in Figs 7–10. It was found that for all waves the dominant harmonic  
 241 of the residue appears to be close to the second harmonic of the force.

242 A separate analysis was conducted by assuming the forces to be described by the  
 243 Morison equation (with time invariant coefficients) corrected by the addition of the  
 244 Lighthill term. Figures 11–14 plot the calculated forces for the full records. When Figs  
 245 11–14 are compared to Figs 3–6 it can be seen that the difference between the respective  
 246 force residues is minimal. This can be seen more clearly in Fig. 15 where the r.m.s.  
 247 errors for the Morison equation are compared with those for the Morison equation  
 248 with the Lighthill correction.

249 We now consider the time histories for individual waves. Figures 16–19 show the  
 250 measured force, the calculated force based on the Morison equation with the Lighthill

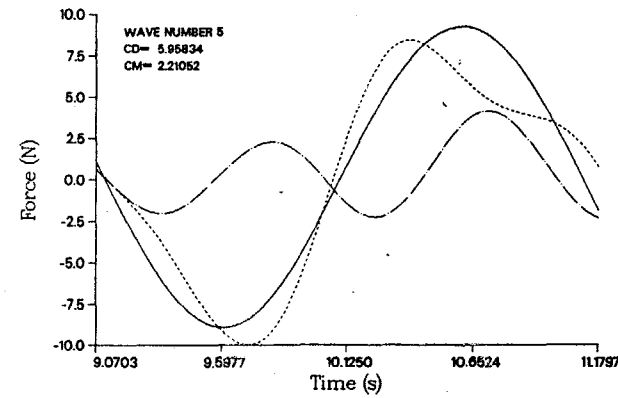


FIG. 7. Individual wave force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 0.32$ .

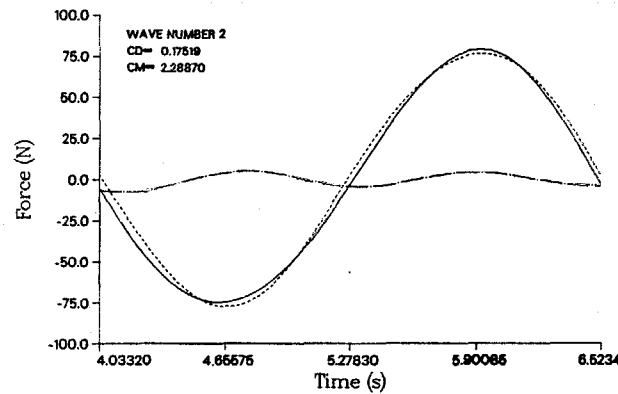


FIG. 8. Individual wave force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 4.41$ .

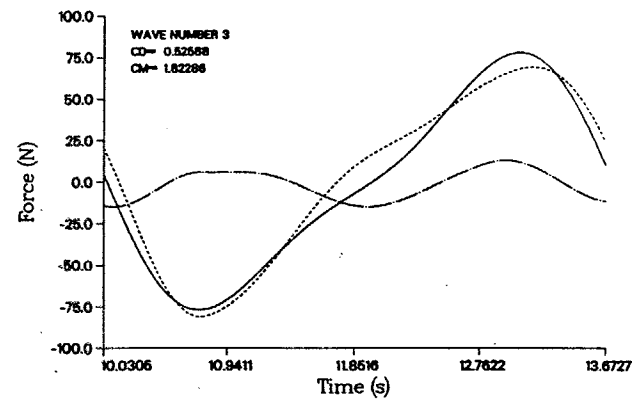


FIG. 9. Individual wave force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 10.26$ .

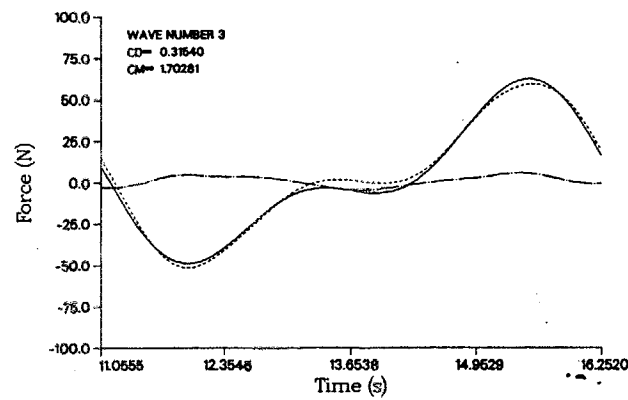


FIG. 10. Individual wave force time histories: measured (—); Morison equation (---); force residue (— · —);  $KC = 15.31$ .

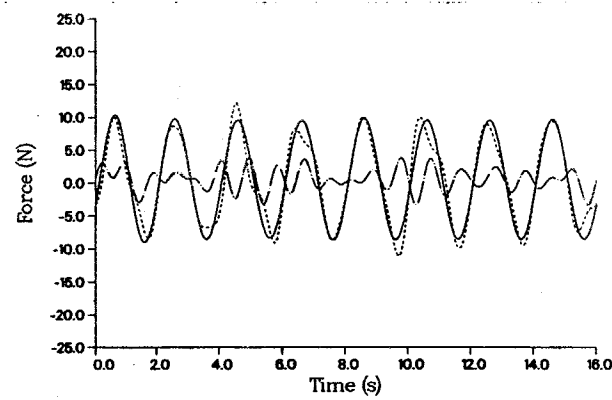


FIG. 11. Force time histories: measured (—); Morison equation with Lighthill correction (---); force residue (— · —);  $KC = 0.32$ .

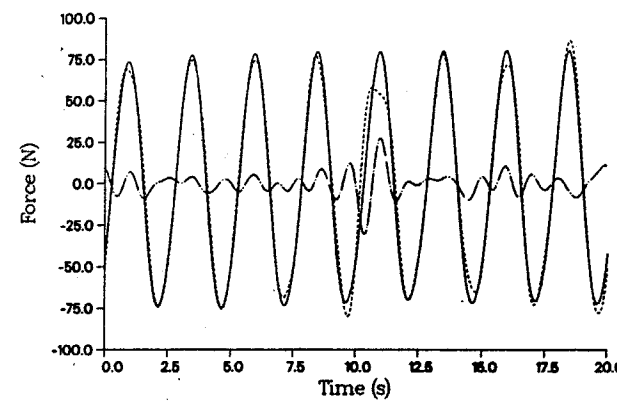


FIG. 12. Force time histories: measured (—); Morison equation with Lighthill correction (---); force residue (— · —);  $KC = 4.41$ .

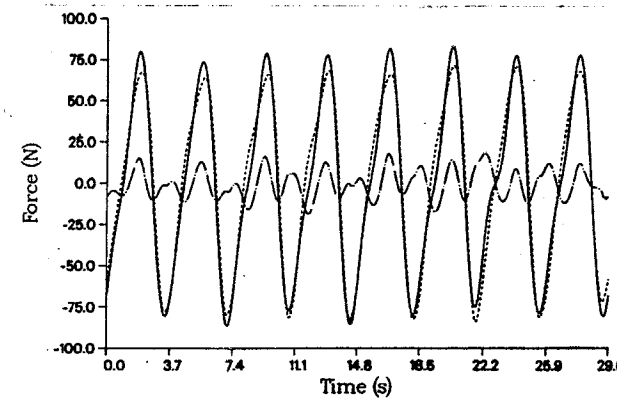


FIG. 13. Force time histories: measured (—); Morison equation with Lighthill correction (---); force residue (— · —);  $KC = 10.26$ .

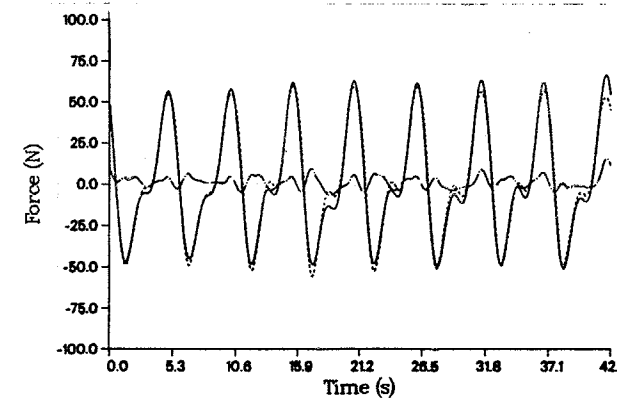


FIG. 14. Force time histories: measured (—); Morison equation with Lighthill correction (---); force residue (— · —);  $KC = 15.31$ .

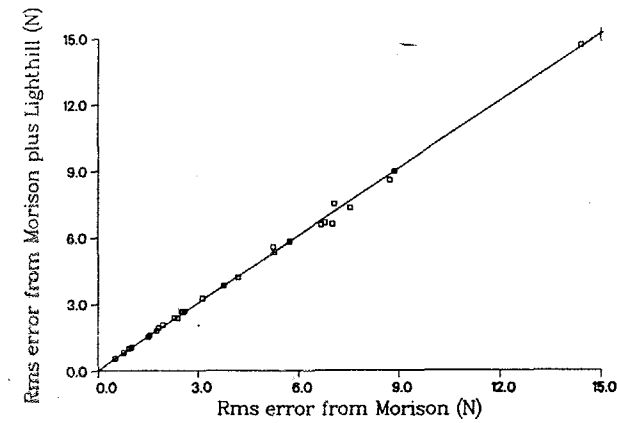


FIG. 15. R.m.s. errors for the time histories obtained by the Morison equation with the Lighthill correction vs the r.m.s. errors for the time histories obtained by the Morison equation.

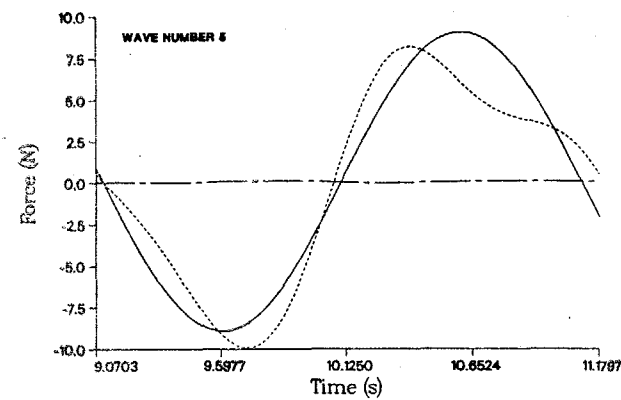


FIG. 16. Individual wave force time histories: measured (—); Morison equation with the Lighthill correction (---); Lighthill correction (— · —);  $KC = 0.32$ .

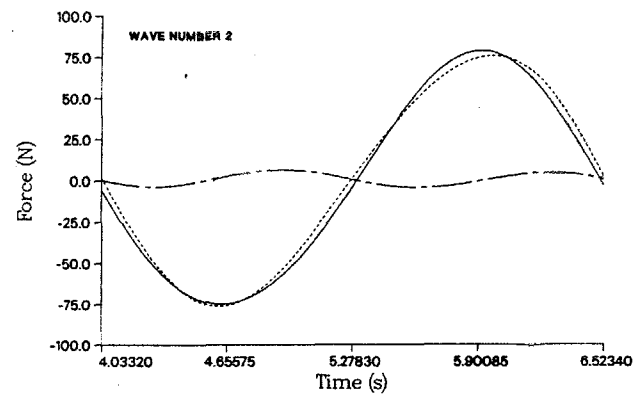


FIG. 17. Individual wave force time histories: measured (—); Morison equation with Lighthill correction (---); Lighthill correction (— · —);  $KC = 4.41$ .

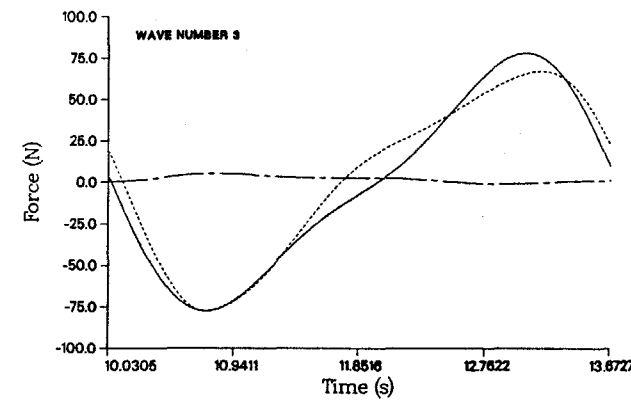


FIG. 18. Individual wave force time histories: measured (—); Morison equation with the Lighthill correction (---); Lighthill correction (— · —);  $KC = 10.26$ .

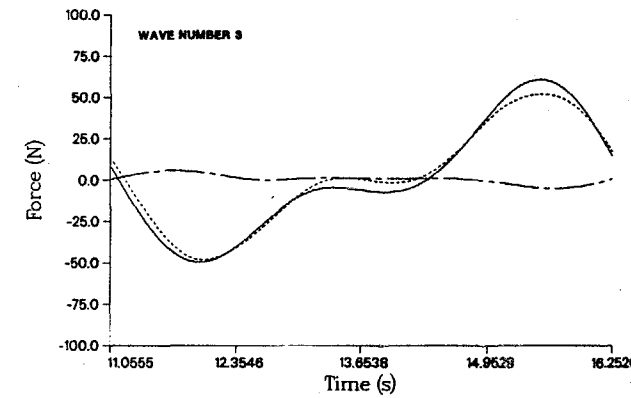


FIG. 19. Individual wave force time histories: measured (—); Morison equation with the Lighthill correction (---); Lighthill correction (— · —);  $KC = 15.31$ .

251 correction and the Lighthill correction itself. (In all figures the Lighthill correction is  
 252 represented by long-dash-short-dash lines.) An overall assessment of the Lighthill  
 253 correction can be made by comparing the r.m.s. error for the Morison equation with  
 254 the Lighthill correction against the r.m.s. error for the Morison equation without  
 255 correction. Figure 20 shows this comparison for each of the waves analysed. It can be  
 256 seen that generally the modeling by the Morison equation is slightly better than the  
 257 modeling by the Morison equation with the Lighthill correction. However, the  
 258 differences are marginal.

259 Calculations were also performed to examine the effect of the Lighthill correction  
 260 on the drag and inertia coefficients. The results can be seen in Figs 21 and 22 where  
 261 the drag and inertia coefficients, respectively, are shown for the individual wave results.  
 262 These two figures plot the force coefficients based on the Morison equation with the  
 263 Lighthill correction (squares), and the force coefficients based on the Morison equation  
 264 (triangles) superimposed. It can be seen that there is very little difference in the drag

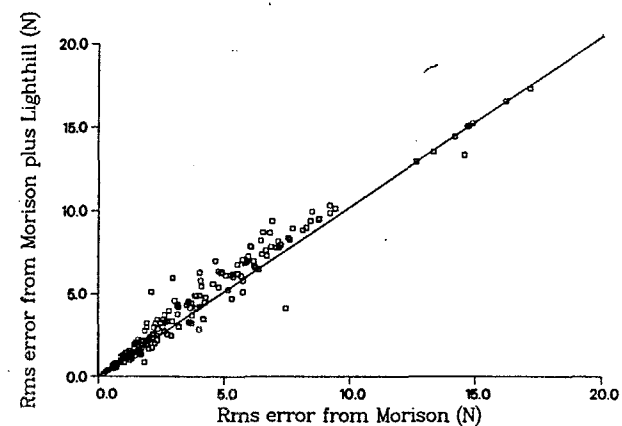


FIG. 20. Rms errors for the time histories obtained by the Morison equation with the Lighthill correction vs the r.m.s. errors for the time histories obtained by the Morison equation.

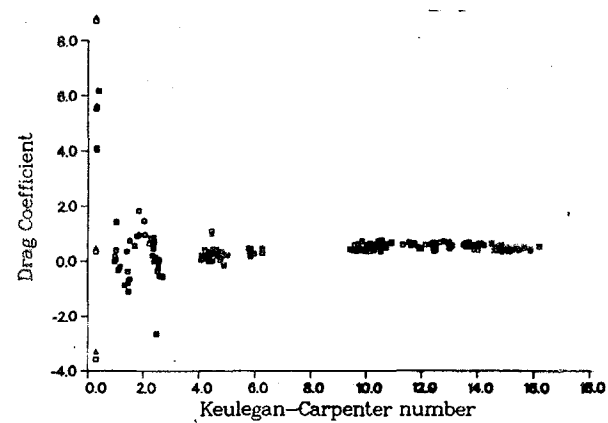


FIG. 21. Comparison of drag coefficients.

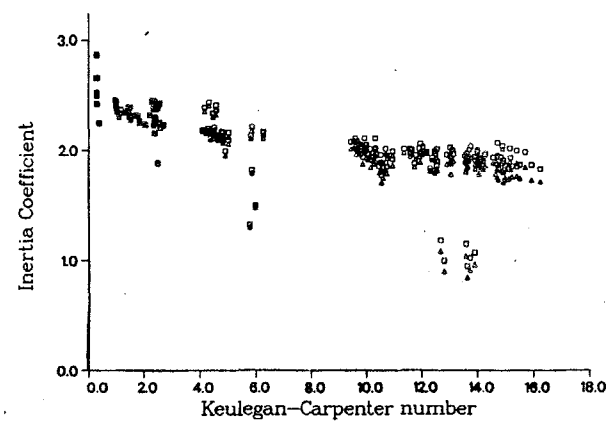


FIG. 22. Comparison of inertia coefficients.

265 coefficients over the whole range of  $KC$  numbers considered. For the inertia coefficients  
 266 the largest error, although still small, occurs at the higher  $KC$  numbers. Hence, it can  
 267 be concluded that for the NCEL data the addition of the Lighthill correction does not  
 268 decrease the drag coefficient significantly.

#### 269 ANALYSIS OF THE RANDOM DATA

270 The data obtained from DHL included the in-line force, the horizontal and vertical  
 271 velocity, all measured at 2.5m (the lower level) and 3.5m (the upper level) above the  
 272 bottom of the tank and wave elevations which were measured upstream and downstream  
 273 of the cylinder.

274 The effective Keulegan-Carpenter numbers as defined by Bishop (1978) ( $KC^* =$   
 275  $(2\pi/0.866D)\sqrt{(u^2/a^2)}$ ) where  $u$  is the velocity,  $a$  is the acceleration and  $D$  is the cylinder  
 276 diameter) where  $KC^* = 5.75$  for the lower level and  $KC^* = 6.0$  for the upper level.  
 277 These low  $KC^*$  values indicate that the dominant part of the total Morison force is due  
 278 to the inertia term. Since the Lighthill correction affects the inertia term, its effect may  
 279 therefore be expected to be most significant in the low  $KC^*$  region.

280 Least squares analyses of the full time histories of the measured forces, velocities  
 281 and accelerations were performed to estimate the drag and inertia coefficients in the  
 282 Morison equation without a correction term. The values so obtained were  $Cd = 0.2345$   
 283 and  $Cm = 1.8295$  for the lower level and  $Cd = 0.5393$  and  $Cm = 2.0502$  for the upper  
 284 level. Force spectra were then calculated using the Morison equation in which these  
 285 coefficients and the measured velocity and acceleration were used. Figures 23 and 24  
 286 show, for the lower and upper levels, respectively, these calculated force spectra (dashed  
 287 line) and the spectra of the measured forces (solid line). It is seen that the Morison  
 288 equation with time invariant coefficients provides an excellent fit to the measured  
 289 forces.

290 Least squares analyses were also performed to obtain the drag and inertia coefficients  
 291 when the Lighthill correction term was included in the calculation of the forces. The  
 292 coefficients obtained from the analyses were  $Cd = 0.2341$ ,  $Cm = 1.8343$  for the lower  
 293 level and  $Cd = 0.5403$ ,  $Cm = 2.0587$  for the upper level. When these coefficients are

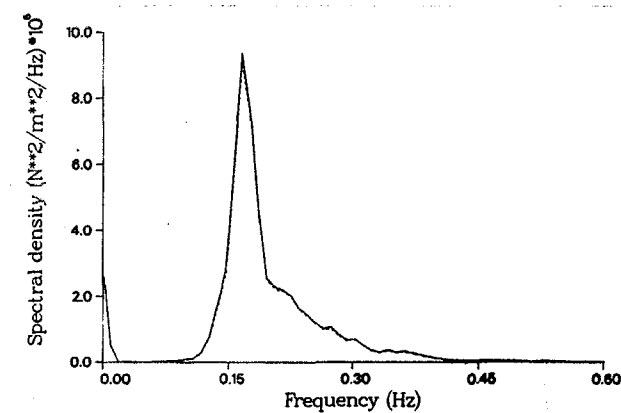


FIG. 23. Spectral density of Morison equation forces and measured forces.

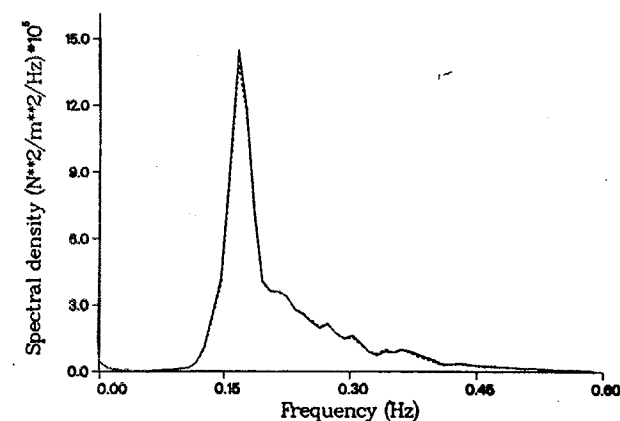


FIG. 24. Spectral density of Morison equation forces and measured forces.

294 compared to those obtained by using the Morison equation without correction it can  
 295 be seen that the effect of the Lighthill term is minimal (of the order of 0.5% or less).  
 296 That the Lighthill term has no significant contribution to the total force is also borne  
 297 out by the measured force spectra (solid line) and the calculated force spectra (dashed  
 298 line) shown in Figs 25 and 26 for the lower and upper levels, respectively. Indeed, it  
 299 can be seen that the addition of the Lighthill correction does not change significantly  
 300 the calculated spectra with respect to their values based on the Morison equation with  
 301 no correction (see Figs 23 and 24).

302 It is concluded that the Morison equation provides an excellent model for the DHL  
 303 measured forces, and that the inclusion of the Lighthill correction term makes no  
 304 significant difference for both the force coefficients and the calculated force spectra.

#### CONCLUSIONS

306 The Morison equation with time invariant coefficients provides an excellent model  
 307 for both the periodic and the random data analysed in this paper. The Lighthill

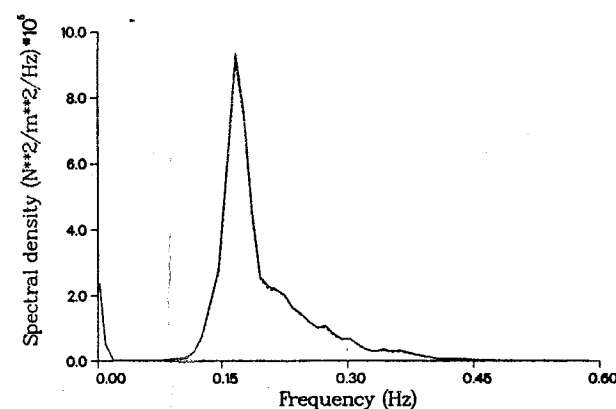


FIG. 25. Spectral density of Morison equation with Lighthill correction forces and measured forces.

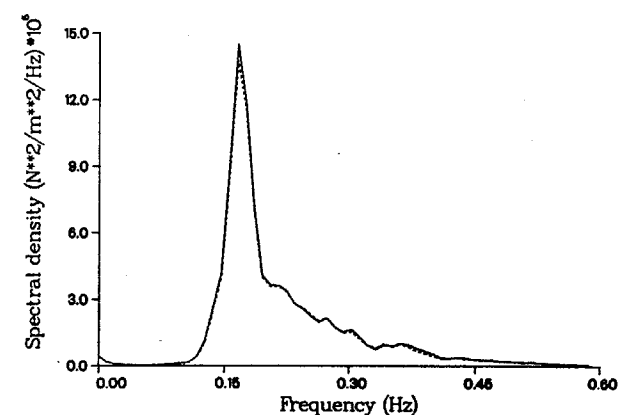


FIG. 26. Spectral density of Morison equation with Lighthill correction forces and measured forces.

308 correction did not improve the performance of the Morison equation and did not alter  
 309 the drag coefficient to any significant extent.

310  
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 315 supplied the periodic data.

#### REFERENCES

- 316  
 318 BEARMAN, P. W., CHAPLIN, J. R., GRAHAM, J. M. R., KOSTENSE, J. K., HALL, P. F. and KLOPMAN, G. 1985a.  
 319 The loading on a cylinder in post-critical flow beneath periodic and random waves. Behaviour of Offshore  
 320 Structures (BOSS '85), The Netherlands.  
 321 BEARMAN, P. W., DOWNIE, M. J., GRAHAM, J. M. R. and OBASAJU, E. D. 1985b. Forces on cylinders in  
 322 viscous oscillatory flow at low Keulegan-Carpenter numbers. *J. Fluid Mechanics* 154, ●●●-●●●.  
 323 BISHOP, J. R. 1978. The mean square value of wave force based on the Morison equation. National Maritime  
 324 Institute Report NMI-R-40.  
 325 CHAKRABARTI, S. 1980. In-line forces on fixed vertical cylinders in waves. *J. WatWays, Port, Coastal Ocean*  
 326 *Div. Am. Soc. civ. Engrs* 106 (WW2), ●●●-●●●.  
 327 CHAKRABARTI, S., WOLBERT, A. and TAM, W. 1976. Wave forces on a vertical cylinder. *J. WatWays, Harbor*  
 328 *Coastal Engng Am. Soc. civ. Engrs* 102 (WW2), ●●●-●●●.  
 329 COOK, G. R. 1987. The Lighthill correction to the Morison equation. Ph.D. dissertation submitted to the  
 330 Johns Hopkins University, Baltimore.  
 331 COOK, G. R., KUMARASENA, T. and SIMIU, E. 1986. Amplification of wind effects on compliant platforms.  
 332 Proceedings of a session at Structures Congress '86, ASCE.  
 333 HUDSPETH, R. T. and NATH, J. H. 1985. High Reynolds number wave force investigation in a wave flume.  
 334 Report No. CR 85.004, Naval Civil Engineering Laboratory, Port Hueneme, CA.  
 335 KEULEGAN, G. H. and CARPENTER, L. H. 1958. Forces on cylinders and plates in an oscillatory fluid. *J. Res.*  
 336 *natn. Bur. Standrds.* 60(5), ●●●-●●●.  
 337 LIGHTHILL, J. 1979. Waves and hydrodynamic loading. *Proc. 2nd Int. Conf. on the Behaviour of Offshore*  
 338 *Structures BOSS '79, London*, Vol. 1.  
 339 MILNE-THOMSON, L. M. 1960. *Theoretical Hydrodynamics*, 4th edn. MacMillan, New York.  
 340 MORISON, J. R., O'BRIEN, M. P., JOHNSON, J. W. and SCHAAF, S. A. 1950. The forces exerted by surface  
 341 waves on piles. *Petroleum Trans.* 189, ●●●-●●●.  
 342 PATERSON, A. R. 1983. *A First Course in Fluid Dynamics*. Cambridge University Press, Cambridge.  
 343 SARPKAYA, T. 1976. In-line and transverse forces on smooth and sand roughened cylinders in oscillatory flow  
 344 at high Reynolds numbers. Report No. NPS-69SL76062, Naval Postgraduate School, Monterey, CA.  
 345 SARPKAYA, T. 1981. Morison equation and the wave forces on offshore structures. Report No. CR 82.008,  
 346 Naval Civil Engineering Laboratory, Port Hueneme, CA.

- 347 SARPKAYA, T. 1986. Force on a circular cylinder in viscous oscillatory flow at low Keulegan-Carpenter  
348 numbers. *J. Fluid Mechanics* **165**, ●●●-●●●.
- 349 SARPKAYA, T. and ISAACSON, M. 1981. *Mechanics of Wave Forces on Offshore Structures*. Van Nostrand  
350 Reinhold, New York.
- 351 SIMIU, E. and LEIGH, S. D. 1983. Turbulent wind effects on compliant platforms. NBS Building Science  
352 Series 151.
- 353 STOKER, J. J. 1957. *Water Waves*. Wiley Interscience, New York.